

CITY OF DIXON, CALIFORNIA

DIXON 257

M&P Project No. 20-0024-00 (v.1)

DRAFT WATER STUDY

July 2023



PREPARED BY:



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- Exhibit 3 – Proposed Well Detail
- Exhibit 4 – Water System Performance and Operational Criteria
- Exhibit 5 – Water Supply Calculations

1. BACKGROUND

This water study provides a preliminary water infrastructure plan and design standards for water facilities within the proposed Dixon 257 project. The project is located on approximately 259.7-acres within a portion of the City of Dixon in Solano County California (APNs 0111-040-010, -020, -030, -040, and 0111-080-050). The project is located within the City of Dixon’s Northeast Quadrant and is part of the Northeast Quadrant Specific Plan (NQSP). The NQSP area is located south of I-80, north of Vaughn Road, east of N. First Street, and west of Pedrick Road. See **Figure 1** for the vicinity map.

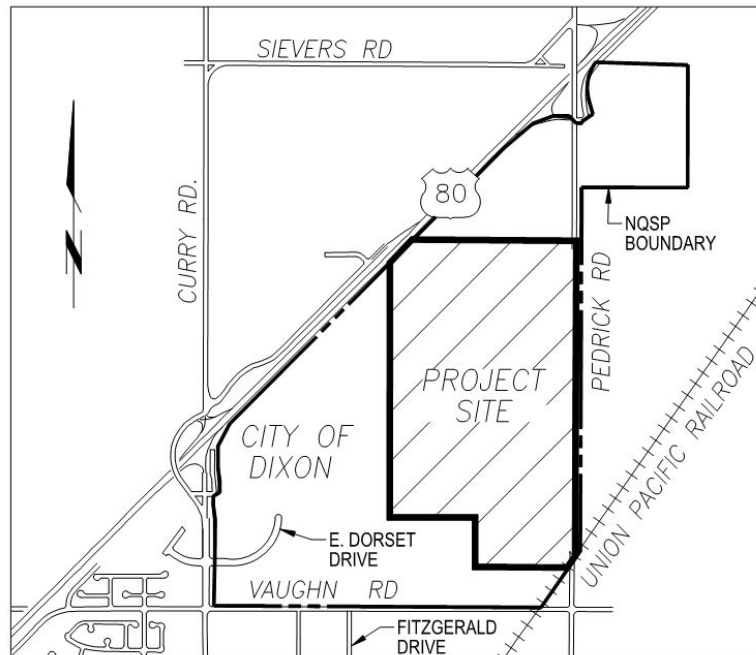


Figure 1 – Vicinity Map

2. PURPOSE

The main objective of this study is to determine the water demands of the Dixon 257 project. This study also seeks to determine the water infrastructure requirements based on the City of Dixon 2016 Water System Master Plan and to provide a preliminary water infrastructure plan that will meet the needs of the proposed site.

3. LAND USE

Historically, the Dixon 257 site has been used for farming. The majority of the site is presently under cultivation with field and row crops with a small portion of the site uncultivated due to the presence of old, concrete building foundations. The existing topography of the site is very flat and generally drains from the west to the east at one-third percent.

The project site is zoned as Corridor Mixed Use (CMU). The site area will be broken up into multiple proposed land uses. The majority of the site, totaling 128.2 acres, will be single-family residential (SFR). The northern portion of the site will contain 46.7 acres of light industrial area (LI), 9.7 acres of multi-family residential (MFR), 2.0 acres of neighborhood commercial (CC),

and a proposed well site on 0.3 acres. The proposed well will be discussed in the following section. See **Exhibit 1** for the Proposed Land Use Mix.

4. CITY OF DIXON WATER SYSTEM MASTER PLAN

4.1. EXISTING INFRASTRUCTURE

The City of Dixon Water System Master Plan (WSMP) by West Yost Associates determined the existing conditions of the Dixon water system at the end of 2016 and recommended water system improvements to meet the needs future development. The City of Dixon’s existing water system is broken up into three zones, the North, South and Core Zones and the Zones are hydraulically connected to each other. The Dixon 257 site lies within the North Zone. The Dixon water system relies completely on groundwater wells. The city has three existing wells, one of which is a standby well for the city, and two storage tanks serving the Core and North Zone service areas. The total capacity of the two operational wells is 3,300 gallons per minute and the total usable volume of the tanks is 1.8 million gallons. Two existing booster pump stations serve the Core and North Zones. Existing 12” water pipelines exist south-west of the project site in East Dorset Drive and to the south of the project in Vaughn Road. **Figure 2** is taken from Figure 2-3 of the WSMP and shows the existing water system in the North Zone near the project site.

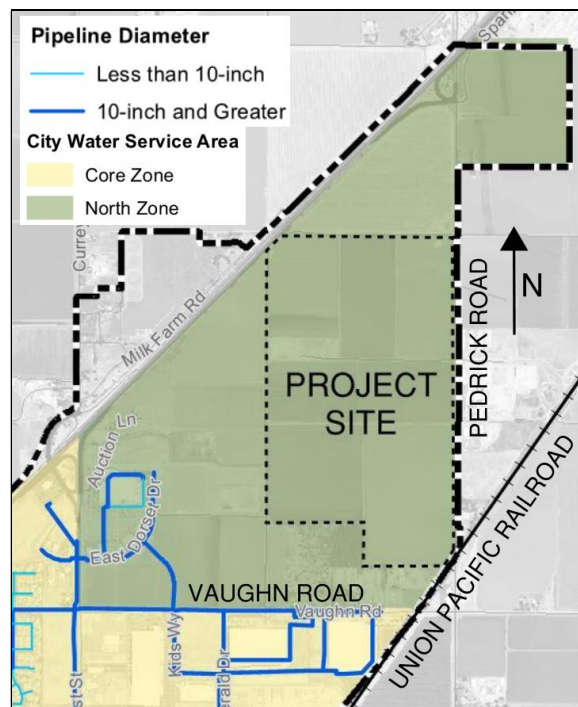


Figure 2 – Existing Water Infrastructure from WSMP

4.2. PROPOSED INFRASTRUCTURE

Figure 3 is taken from Figure 8-1 of the WSMP and shows proposed future water facilities in the North Zone. The WSMP proposes construction of a new 1,500 gallon per minute well in the Northeast Quadrant (North Zone) by 2030. In future buildout conditions, an additional well and 0.26 MG of useable storage are proposed within the Northeast Quadrant (North Zone). Construction of a new 1,500 gpm well is proposed as part of the Dixon 257 project and will be

located in the northwest portion of the site. The proposed well site can accommodate a future storage tank and an additional well will be constructed within the Northeast Quadrant in future build-out conditions when deemed necessary by the City of Dixon. The future second well site will tentatively be located at the northeast edge of the specific plan. In addition to the proposed well, **Figure 3** shows proposed 12” water mains serving the site and the parcels north with two connections in East Dorset Drive and two connections in Vaughn Road.

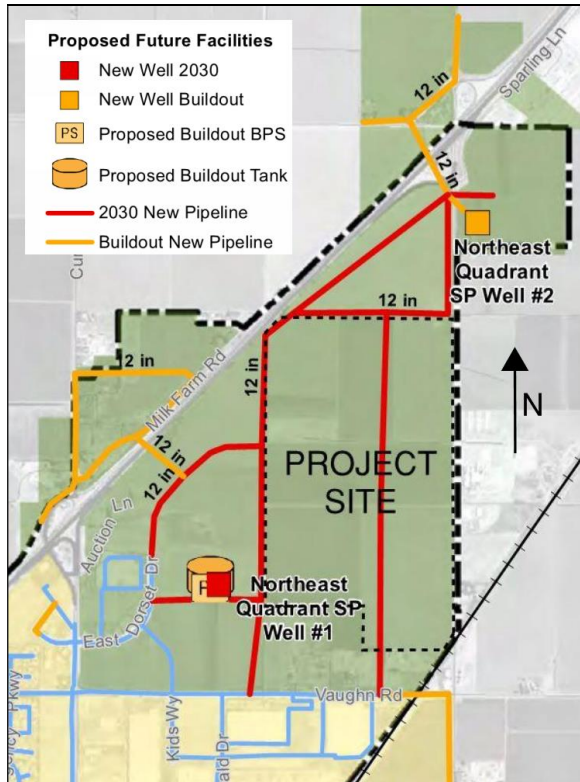


Figure 3 – Proposed Water Infrastructure per the WSMP

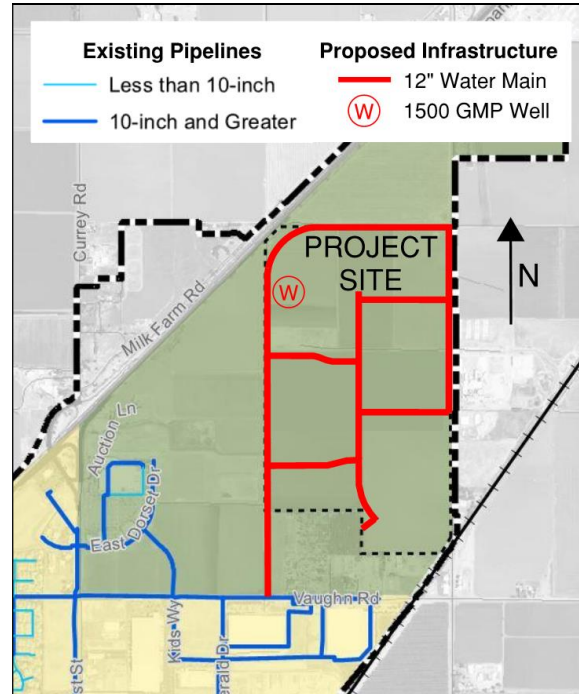


Figure 4 – Proposed Water Infrastructure for the Dixon 257 Project

Figure 4 shows the water infrastructure proposed as part of the Dixon 257 project. Note that the proposed water main alignments and well location differ from **Figure 3**. The Dixon 257 project will follow the intention of the WSMP by providing a new well within the North Zone and extending the water distribution system up to the northern boundary of parcel. However, the water main alignments will differ from the WSMP and only one connection will be provided to Vaughn Road rather than two. The second connection for the first phase of development will be at the well site.

Exhibit 2 is the Water Plan and Shed Map. It shows the water infrastructure proposed as part of the Dixon 257 project in more detail than **Figure 4**, as well as the water supply sheds which will be discussed in Section 6. The proposed well will be based on Detail DF-6 from the Dixon Engineering Design Standards. This detail is provided as **Exhibit 3**.

5. DISTRIBUTION AND FIRE FLOW REQUIREMENTS

Exhibit 4 is a summary of water system performance and operational criteria taken directly from the WSMP. The WSMP provides requirements for fire flow, water supply capacity, water distribution system capacity, pumping facility capacity and water storage capacity. It also has requirements for system pressure, pipe diameter, pipe material and flow velocity. Pipelines serving multi-family residential, commercial, and industrial developments shall be a minimum of 12 inches in size. **Table 1** provides minimum and maximum distribution system pressures.

Distribution System Pressures	
Minimum Pressure – Normal Operating Conditions	50 psi
Minimum Pressure – Peak Hour Conditions	45 psi
Minimum Pressure – Fire Flow Conditions	20 psi
Maximum Pressure	80 psi

Table 1 – Distribution System Pressures

Table 2 provides the fire flow requirements that pertain to the Dixon 257 project, based on Table 5-4 from the WSMP. The project lies within the Northeast Quadrant (NEQ) which has larger fire flow requirements than other areas within the city. For planning purposes, fire flows are assumed to be met concurrently with a Maximum Day Demand condition, while maintaining a residual system pressure of 20 psi throughout the City of Dixon’s service area. Calculation of Maximum Day Demand is discussed in Section 6.

Fire Flow Requirements for New Developments within the NEQ		
Land Use	Flow, gpm	Duration, hours
Single-Family Residential	1,000	3
Multi-Family Residential	2,500	3
Commercial in the Northeast Quadrant (NEQ)	4,000	3
Industrial in the NEQ and Future Areas East of Railroad Tracks	4,000	3
Note 1: Unique projects or projects with alternate materials may require higher fire flow and should be reviewed by the Fire Marshal on a case-by-case basis.		
Note 2: Fire flows are to be supplied at a minimum residual pressure of 20 psi.		

Table 2 – Fire Flow Requirements for New Development within the NEQ

6. WATER DEMANDS

According to the WSMP, the City of Dixon’s peak water supply capacity is sized to meet Maximum Day Demand for each of its zones. It must also be able to meet four hours of Peak Hour Demand with source capacity, storage capacity, and/or emergency source connections. To determine how the Dixon 257 project will affect the City of Dixon’s ability to meet these demand requirements, it is necessary to estimate the Maximum Day Demand and Peak Hour Demand for the site.

Maximum Day Demand and Peak Hour Demand have been calculated based on the design criteria from Chapter 5 of the WSMP. Per Section 5.1.1 of the WSMP, Average Day Demand is calculated by multiplying area by the Unit Demand Factor, which is dependent on land use. Per

Section 5.1.2, Maximum Day Demand is 2.2 times Average Day Demand and Peak Hour Demand is 3.3 times Average Day Demand. **Table 3** shows the unit water demand factors used as part of this study. The table is based on Table 5-3 from Section 5.1.1 of the WSMP.

Water Unit Demand Factors	
Land Use	Unit Demand Factor, af/ac/yr
Single-Family	2.7
Multi-Family	3.9
Commercial/Public	1.3
Industrial	1.5
Landscape	3.0
Maximum Day Demand Factor: 2.2 times Average Day Demand	
Peak Hour Demand Factor: 3.3 times Average Day Demand	

Table 3 – Water Unit Demand Factors from the WSMP

Water demands were calculated based on the land uses shown on the **Exhibit 1** Proposed Land Use Mix and the **Table 3** Unit Demand Factors. The total on-site water supply shed area is 259.6 acres. The proposed well site was assumed to have water demands similar to that of an industrial site. Roadway areas were assumed to have no water demand. **Exhibit 2** shows the water supply sheds as well as proposed water infrastructure. **Exhibit 5** provides the full water demand calculation. **Table 4** summarizes the results of the water demand calculation.

Project Water Demands				
Project Area (ac.)	Average Water Demand (af/yr)	Average Day Demand (mgd)	Maximum Day Demand (mgd)	Peak Hour Demand (mgd)
259.6	562.7	0.502	1.105	1.658

Table 4 – Project Water Demands

7. CONCLUSIONS

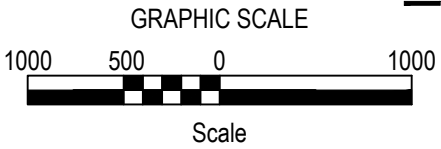
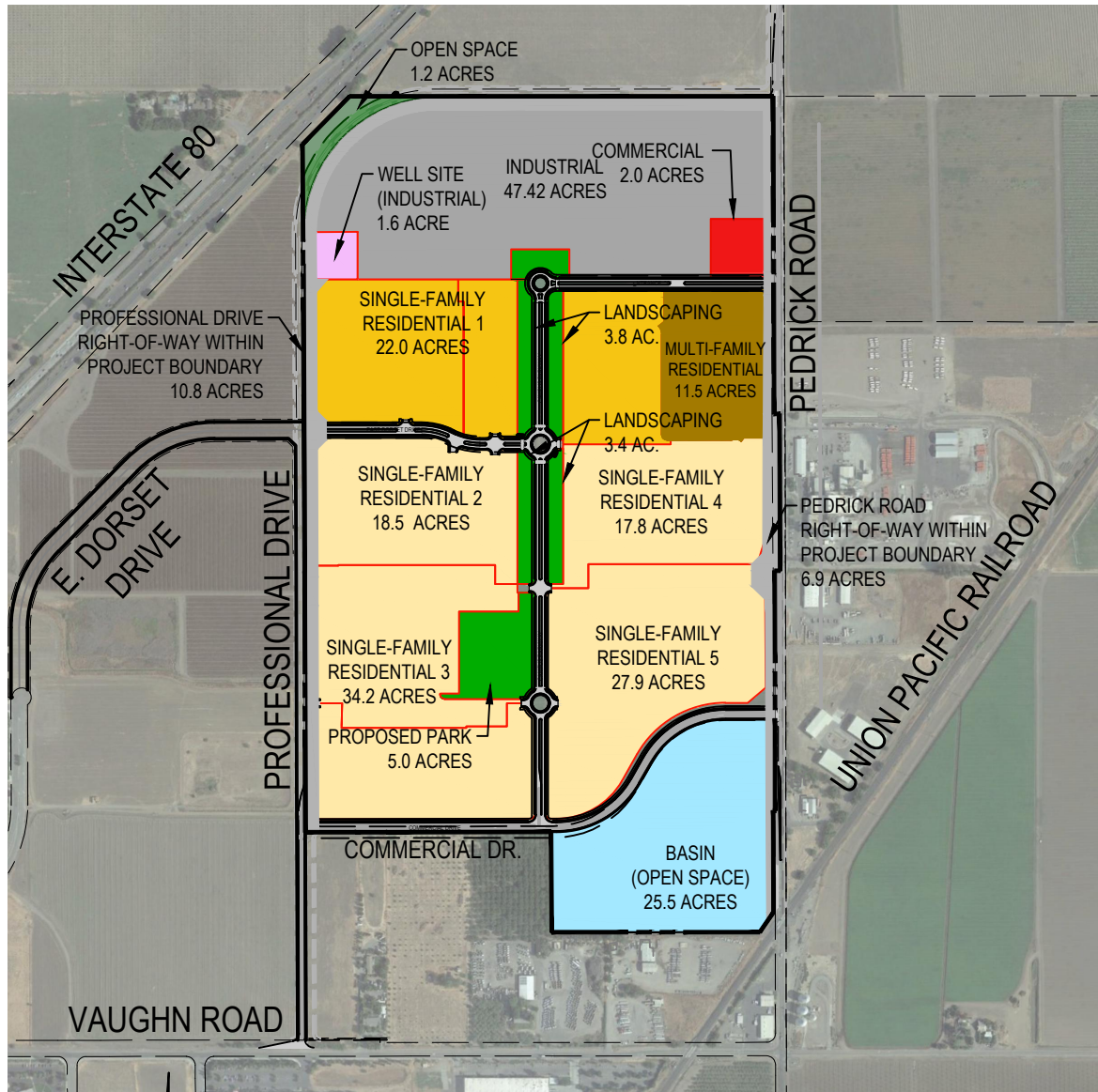
The Dixon 257 project will extend the City of Dixon’s water system northeastward with connections in East Dorset Drive and Vaughn Road. The project will also provide a 1500 gallon per minute well in the northwest corner of the project. Using the methodology from the WSMP, this study estimated average annual water demand for the project to be 562.7 acre-feet. Maximum Day Demand and Peak Hour Demand were estimated to be 1.105 and 1.658 million gallons per day respectively. In the future, an additional well and 0.26 MG of storage will be constructed within Dixon’s North Zone. The well site being proposed can accommodate a future tank. The future second well site will tentatively be located at the northeast edge of the specific plan.

This is a preliminary study of proposed water infrastructure. Further analysis must be conducted to confirm that proposed water infrastructure improvements will meet the performance and operational criteria of the City of Dixon’s Water System Master Plan. This includes modeling the proposed system to ensure that minimum system pressure is maintained during fire flow conditions.

8. REFERENCES

1. City of Dixon Engineering Design Standards, 2022
2. City of Dixon 2016 Water System Master Plan and Strategic Asset Management Plan, West Yost, 2016

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LEGEND

- EXISTING ROADWAYS
- FUTURE ROADWAYS
- PROJECT BOUNDARY

LAND USE TABLE

LAND USE	ACRES
SINGLE-FAMILY RESIDENTIAL	120.1
MULTI-FAMILY RESIDENTIAL	11.5
INDUSTRIAL	47.4
NEIGHBORHOOD COMMERCIAL	2.0
WELL SITE	1.6
ROADS, OPEN SPACE AND BASIN	63.6
PARK AND LANDSCAPING	13.4
TOTAL SITE AREA	259.6

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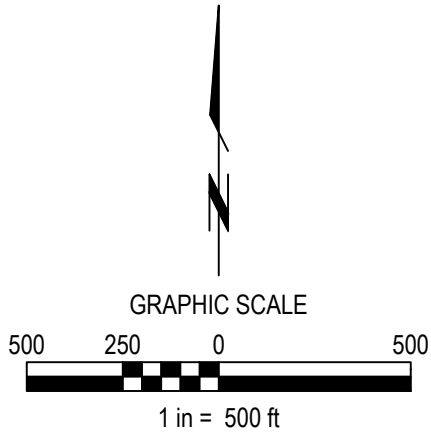
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EXHIBIT 1

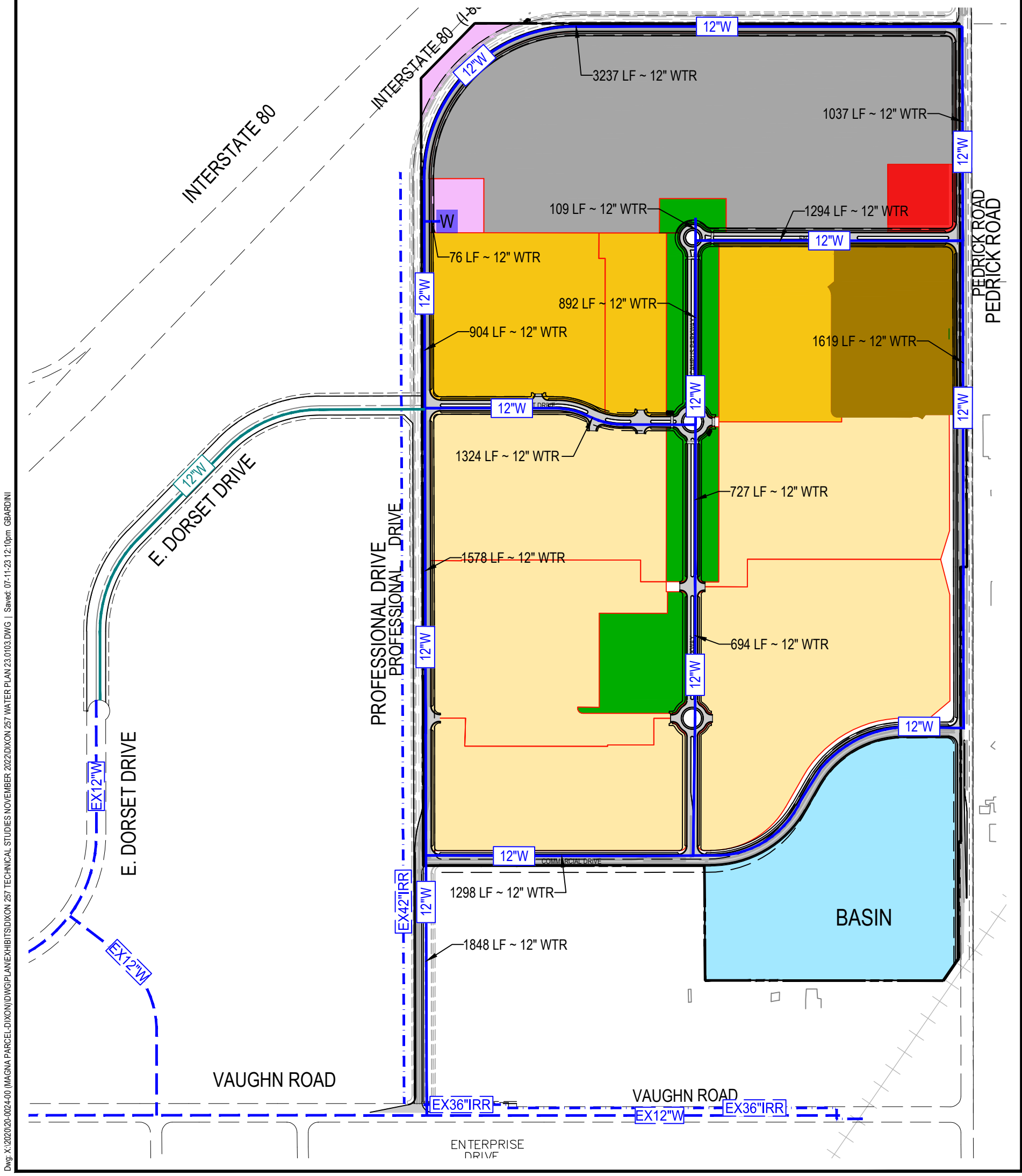
DIXON 257 WATER STUDY

PROPOSED LAND USE MIX

DIXON, CALIFORNIA



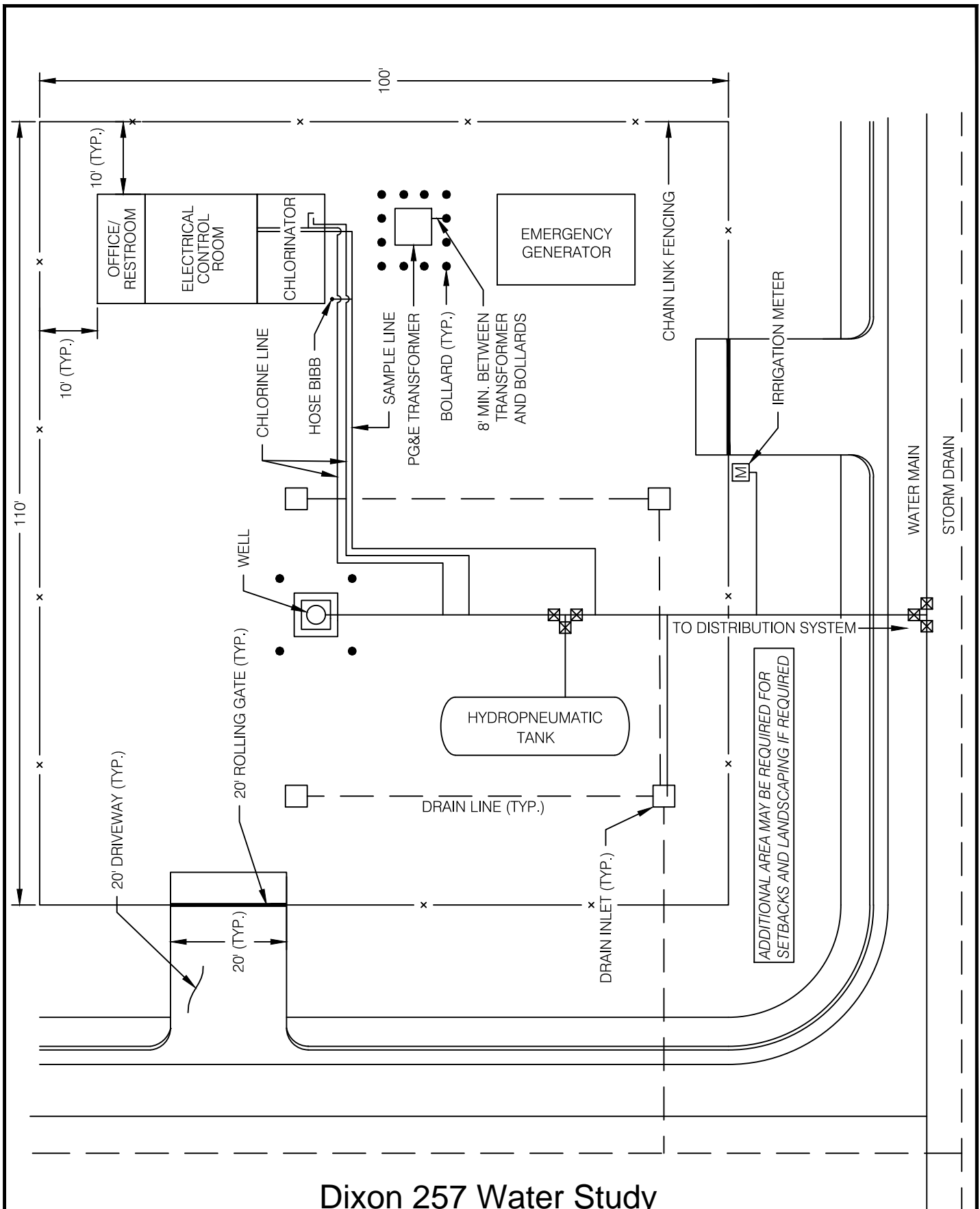
LEGEND			
	SINGLE-FAMILY (SFR) LAND USE		EXISTING ROADWAYS
	MULTI-FAMILY (MFR) LAND USE		FUTURE ROADWAYS
	LIGHT INDUSTRIAL (LI) LAND USE		PROJECT BOUNDARY
	COMMERCIAL (CC) LAND USE		WATER SUPPLY SHED BOUNDARY
	LANDSCAPING AND OPEN SPACE		PROPOSED WELL SITE
	ROADWAYS		PROPOSED WATER MAIN
			EXISTING WATER MAIN
			EXISTING IRRIGATION MAIN
			FUTURE WATER MAIN



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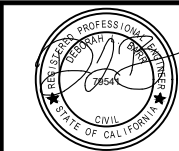
EXHIBIT FOR
DIXON 257 WATER STUDY
EXHIBIT 2
WATER PLAN
 DIXON, CA



Dixon 257 Water Study
 Exhibit 3 - Proposed Well Detail

DF-6

TYPICAL DIRECT
 FEED WELL
 FACILITY



APPROVED: MARCH 2022

CITY OF DIXON
 ENGINEERING
 STANDARD DETAIL



Table 3. Summary of Recommended Potable Water System Performance and Operational Criteria		
Component	Criteria	Remarks / Issues
Fire Flow Requirements (flow [gpm] @ duration [hours]) ^(a)		
Single Family Residential	1,000 gpm @ 3 hours	
Multi Family Residential	2,500 gpm @ 3 hours	
Commercial and Industrial	3,500 gpm @ 3 hours	Includes schools
Commercial and Industrial in Northeast Quadrant	4,000 gpm @ 3 hours	
Water Supply Capacity ^(b)		
Supply / Pumping Capacity	Provide firm supply capacity equal to maximum day demand	Firm groundwater supply capacity is defined as the largest facility out of service for maintenance
Water Distribution System Capacity ^(b)		
Maximum Day Demand plus Fire Flow	Provide firm capacity equal to maximum day demand plus fire flow	
Peak Hour Demand	Provide firm capacity equal to peak hour demand	
Pumping Facility Capacity ^(b)		
Pumping Capacity	Provide the greater of maximum day concurrent with fire flow or peak hour demand	Assumes firm pumping capacity. Firm pumping capacity is defined as the total booster pump station capacity with the largest pump out of service
Backup Power	Provide backup power at all wells and pump stations	
Water Storage Capacity ^(c)		
Operational	20 percent of maximum day demand	
Fire	Largest fire flow for each zone	North/Core Zone: 4000 gpm x 3 hrs = 0.72 million gallons (MG) South Zone: 3500 gpm x 3 hrs = 0.63 MG
Emergency	1 x average day demand (minimum)	Provided by the City's backup power at all pumping facilities
Total Water Storage Capacity	Operational + Fire + Emergency	
Distribution System Pressures ^(d)		
Minimum Pressure - Normal Operating Conditions	50 psi	
Minimum Pressure - Peak Hour Conditions	45 psi	
Maximum Pressure	80 psi	
Minimum Pressure - Fire Flow Conditions	20 psi	At all customer service connections
Water Transmission and Distribution Pipelines ^(a)		
Minimum Pipeline Diameter	6-inch; 12-inch for multi-family residential, commercial, and industrial developments with more than two units	Locate new distribution pipelines within designated utility corridors wherever possible
Maximum Velocity - Normal Operating Conditions	6 ft/s	Criteria based on requirements for new development. Existing distribution mains will be evaluated on case-by-case basis. Evaluation will include age, material, type, velocity, headloss and pressure
Maximum Velocity - Fire Flow Conditions	12 ft/s	
Hazen Williams "C" Factor	130	For consistency in hydraulic modeling
Pipeline Material	Polyvinyl chloride (PVC)	For consistency in hydraulic modeling
<p>(a) Criteria based on the City's Engineering Design Standards, August 2014 Section 5.</p> <p>(b) Criteria included in the City's 2000 Master Plan</p> <p>(c) Water storage capacity fire component criteria updated to provide redundancy in the North/Core and South Zones</p> <p>(d) Criteria based on SCADA data and actual system operation pressures.</p>		

Morton & Pitalo, Inc

Project: Dixon 257 Water Study
Job No. 20-0024-00
Date: 1/4/2023

References: *City of Dixon Engineering Design Standards (Mar. 2022),
 City of Dixon 2016 Water System Master Plan and Strategic Asset Management Plan*

Assumptions:

- 1. Proposed well site is assumed to be part of the Industrial land use

Table 3-6. Recommended Unit Water Demand Factors	
Water Use Type	Unit Demand Factor, af/ac/yr
Single Family Residential (SFR)	2.7
Multi-Family Residential (MFR)	3.9
Commercial	1.3
Industrial	1.5
Landscape	3.0

*Table from page 3-12 of 2016 Water System Master Plan

Table 3-8 Peaking Factors	
Maximum Day Demand Factor	2.2 times average day demand
Peak Hour Demand Factor	3.3 times average day demand

*Table from page 3-13 of 2016 Water System Master Plan

Design Flows

Name of Proposed Land Use	Area (Ac.)	Unit Water Demand Factor (af/ac/yr)	Average Water Demand (af/yr)	Average Day Demand (mgd)	Maximum Day Demand (mgd)	Peak Hour Demand (mgd)
Industrial	46.7	1.5	70.1	0.063	0.138	0.206
Well Site	0.3	1.5	0.5	0.000	0.001	0.001
Commercial	2.0	1.3	2.6	0.002	0.005	0.008
SFR 1	20.0	2.7	54.0	0.048	0.106	0.159
SFR 2	20.2	2.7	54.5	0.049	0.107	0.161
SFR 3	30.9	2.7	83.4	0.074	0.164	0.246
SFR 4	28.3	2.7	76.4	0.068	0.150	0.225
SFR 5	28.8	2.7	77.8	0.069	0.153	0.229
MFR	9.7	3.9	37.8	0.034	0.074	0.111
Park	5.5	3.0	16.5	0.015	0.032	0.049
Landscaping	9.0	3.0	27.0	0.024	0.053	0.080
Basin & O. Space	20.7	3.0	62.1	0.055	0.122	0.183
Roadways	37.5	0	0	0	0	0
Totals:	259.6		562.7	0.502	1.105	1.658